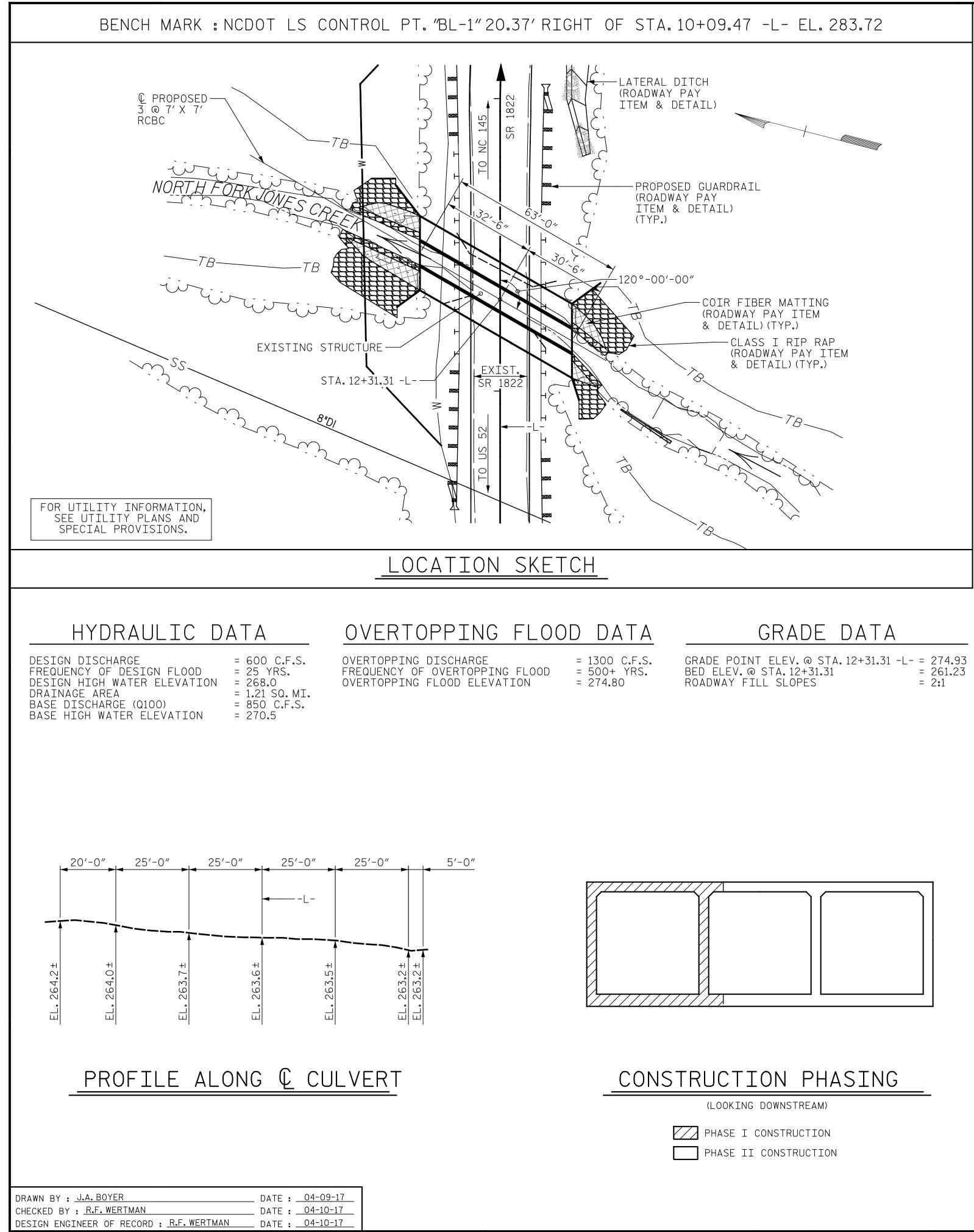
# This electronic collection of documents is provided for the convenience of the user and is Not a Certified Document –

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GRADE POINT ELEV. @ STA. 12+31.31 -L-	=	274.93
BED ELEV. @ STA. 12+31.31	=	261.23
ROADWAY FILL SLOPES	Ξ	2:1

ASSUMED LIVE LOAD ----- HL-93 OR ALTERNATE LOADING. THIS CULVERT IS LOCATED IN SEISMIC ZONE 1.

DESIGN FILL------4'-9"MINIMUM AND 6'-11"MAXIMUN

FOR OTHER DESIGN DATA AND NOTES SEE STANDARD NOTE SHEE

3"Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS.

CONCRETE IN CULVERTS TO BE POURED IN THE FOLLOWING ORD

- 1. PHASE I WING FOOTINGS AND FLOOR SLAB INCLUDING 4"
- OF ALL VERTICAL WALLS. 2. THE REMAINING PORTIONS OF THE WALLS AND WINGS FULL HEIGHT FOLLOWED BY ROOF SLAB AND HEADWALLS.
- 3. PHASE II WING FOOTINGS AND FLOOR SLAB INCLUDING 4" OF ALL VERTICAL WALLS.
- 4. THE REMAINING PORTIONS OF THE WALLS AND WINGS FULL HEIGHT FOLLOWED BY ROOF SLAB AND HEADWALLS.

THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

DIMENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL EMBEDDED IN BARREL ARE SHOWN ON WING SHEET.

A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO TH FILL FACE OF THE WING COVERING THE ENTIRE LENGTH OF THE EXPANSION JOINT.

STEEL IN THE BOTTOM SLAB MAY BE SPLICED AT THE PERMITT CONSTRUCTION JOINT AT THE CONTRACTOR'S OPTION. EXTRA WE OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CULVERT DIVERSION DETAILS AND PAY ITEM, SEE EROSION CONTROL PLANS.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIC

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL IN THE INTERIOR FACE OF EXTERIOR WAL BOTH FACES OF INTERIOR WALLS ABOVE LOWER WALL CONSTRUC JOINT. THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPL LENGTH CHART SHOWN ON THE PLANS.EXTRA WEIGHT OF STEEL TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.

AT THE CONTRACTOR'S OPTION HE MAY SUBMIT, TO THE ENGINEE FOR APPROVAL, DESIGN AND DETAIL DRAWINGS FOR A PRECAST REINFORCED CÓNCRETE BOX CULVERT IN LIEU OF THE CAST-IN CULVERT SHOWN ON THE PLANS. THE DESIGN SHALL PROVIDE THE SAME SIZE AND NUMBER OF BARRELS AS USED ON THE CAST-IN-DESIGN.FOR OPTIONAL PRECAST REINFORCED CONCRETE BOX CUL SEE SPECIAL PROVISIONS.

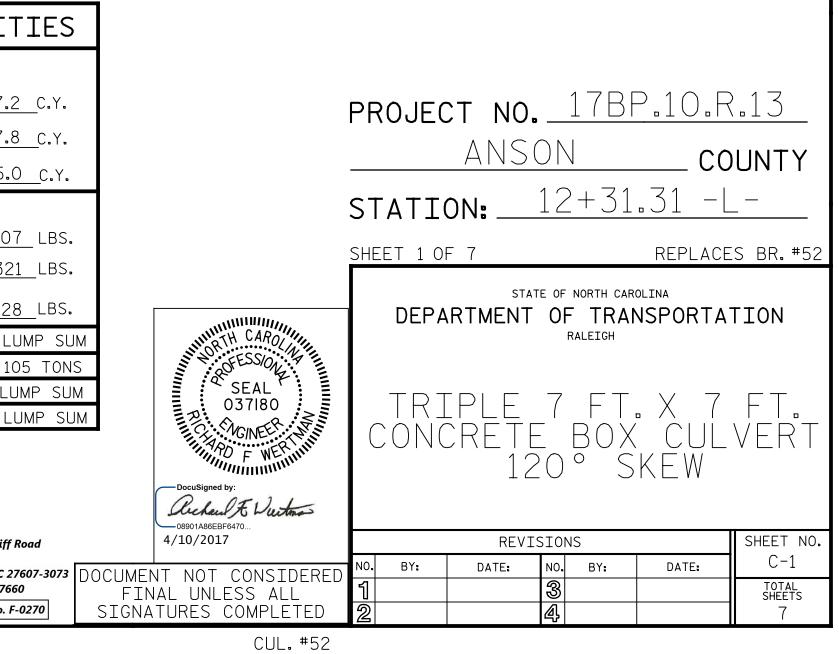
TOTAL	STRUC	TURE	QUANTII
CLASS A CO	ONCRETE		
BARREL @_	2.019	CY/FT_	127.2
WINGS,ETC	B		27.8
REINFORCI	NG STEEL		
BARREL			2050
WINGS,ETC	D		132
TOTAL			21828
CULVERT E	XCAVATION	*	Ll
FOUNDATIO	N COND.MA	T'L	10
REMOVAL O	F EXISTIN	G STRUCI	URE LL
ASBESTOS	ASSESSMEN	Τ	Ll
* INCLUDES	S CLASS I	I SELECT	BACKFILL



Suite 102

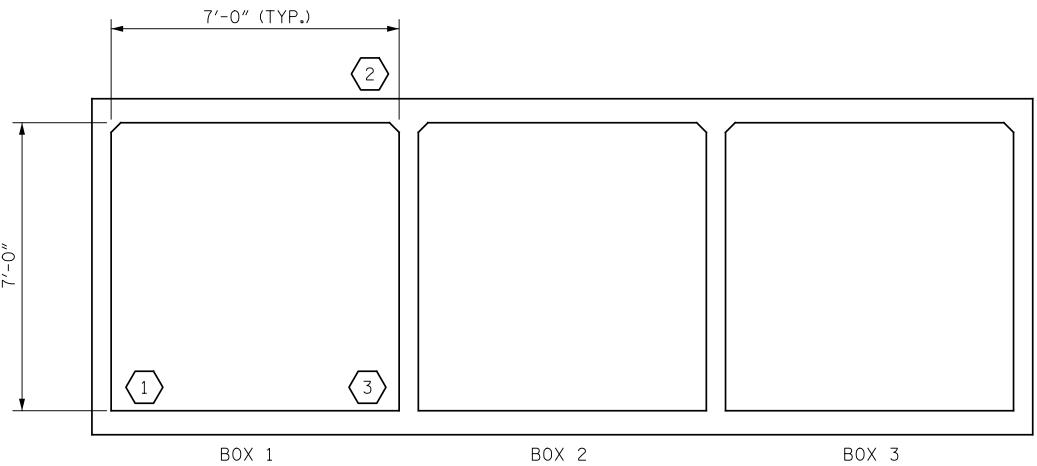
## NOTES

И Т.	THE EXISTING 1SPAN STRUCTURE (1 @ 16'-10") WITH A CLEAR ROADWAY WIDTH OF 19.1' ON A TIMBER DECK WITH 10 LINES OF 5X12" TIMBER JOISTS ON A SUBSTRUCTURE CONSISTING OF YOUNT MASONRY ABUTMENTS AT THE SITE OF THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED BELOW THE LEGAL LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE FURTHER DETERIORATE, THIS LOAD LIMITATION MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.
ER:	THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE.SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.
	REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.
	FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
	BACKFILL WITH SELECT MATERIAL, CLASS II MEETING THE REQUIREMENTS OF SECTION 1016 OF THE STANDARD SPECIFICATIONS.
HE	EXCAVATE AT LEAST 1 FOOT BELOW BOTTOM OF CULVERT AND REPLACE WITH FOUNDATION CONDITIONING MATERIAL IN ACCORD WITH ARTICLE 414-4 OF THE STANDARD PROVISION.
ED IGHT	MATERIAL EXCAVATED FROM THE EXISTING BED SHALL BE STOCKPILED FOR USE IN THE PROPOSED CULVERT BARRELS.UPON COMPLETION OF THE PROPOSED CULVERT, THE MATERIAL SHALL BE PLACED IN THE CENTER BARREL TO A DEPTH OF 1'-O" AND IN THE OUTER BARRELS BETWEEN SILLS TO A DEPTH OF 2'-O". BED MATERIAL MAY BE SUPPLEMENTED WITH CLASS I RIP RAP IF SUITABLE MATERIAL IS NOT AVAILABLE IN SUFFICIENT QUANTITIES.
	THE ENTIRE COST OF WORK REQUIRED TO PLACE THE EXCAVATED MATERIAL OR SUPPLEMENTAL MATERIAL SHALL BE INCLUDED IN THE CONTRACT LUMP SUM PRICE BID FOR CULVERT EXCAVATION.
INS.	THE REQUIRED BEARING CAPACITY (FACTORED RESISTANCE)AT THE BASE OF THE CULVERT IS 1.5 TSF. THE REQUIRED BEARING CAPACITY SHALL BE VERIFIED.
	NO PRECAST REINFORCED BOX CULVERT OPTION WILL BE ALLOWED.
_ AND TION .ICE DUE	FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.
ER	
-PLACE	
E -PLACE LVERT,	



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			SUMM.													<u> </u>
										STRENGTH	I LIM	MIT STATE				
										MOMENT				SHEAR		
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD Factors (Y <sub>LL</sub> )	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (f†)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (f†)	
		HL-93 (INVENTORY)	NZA	$\langle 1 \rangle$	1.22		1.75	1.68	1	EXTERIOR WALL	6.67	1.22	1	BOTTOM SLAB	6.45	
DESIGN		HL-93 (OPERATING)	N/A		1.58		1.35	2.18	1	EXTERIOR WALL	6.67	1.58	1	BOTTOM SLAB	6.45	
LOAD RATING		HS-20 (INVENTORY)	36.000	$\langle 2 \rangle$	2.00	72.00	1.75	2.01	1	EXTERIOR WALL	6.67	2.00	1	TOP SLAB	6.20	
		HS-20 (OPERATING)	36.000		2.59	93.33	1.35	2.61	1	EXTERIOR WALL	6.67	2,59	1	TOP SLAB	6.20	
		SNSH	13.500		3.13	42.26	1.40	3.13	1	EXTERIOR WALL	6.67	3.27	1	TOP SLAB	6.20	
	ш	SNGARBS2	20.000		2.94	58.80	1.40	2.94	1	EXTERIOR WALL	6.67	3.04	1	TOP SLAB	6.20	
	ICL	SNAGRIS2	22.000		3.13	68.86	1.40	3.13	1	EXTERIOR WALL	6.67	3.20	1	TOP SLAB	6.20	
	<pre> </pre>	SNCOTTS3	27.250	$\overline{3}$	1.73	47.14	1.40	2.27	1	EXTERIOR WALL	6.67	1.73	1	BOTTOM SLAB	6.45	
		SNAGGRS4	34.925		1.95	68.10	1.40	2.36	1	BOTTOM SLAB	7.00	1.95	1	BOTTOM SLAB	6.45	
	SINGLE	SNS5A	35.550		2.28	81.05	1.40	2.66	1	BOTTOM SLAB	7.00	2.28	1	BOTTOM SLAB	6.45	
		SNS6A	39.950		2.42	96.68	1.40	2.82	1	BOTTOM SLAB	7.00	2.42	1	BOTTOM SLAB	6.45	
LEGAL LOAD		SNS7B	42.000		2.53	106.26	1.40	2.94	1	BOTTOM SLAB	7.00	2.53	1	BOTTOM SLAB	6.45	
RATING	LER	TNAGRIT3	33.000		2.50	82.50	1.40	3.01	1	BOTTOM SLAB	7.00	2.50	1	TOP SLAB	6.20	
	TRAI	TNT4A	33.075		2.03	67.14	1.40	2.66	1	EXTERIOR WALL	6.67	2.03	1	BOTTOM SLAB	6.45	
	L-IM	TNT6A	41.600		2.10	87.36	1.40	2.55	1	BOTTOM SLAB	7.00	2.10	1	BOTTOM SLAB	6.45	
	ST)	TNT7A	42.000		2.30	96.60	1.40	2.79	1	BOTTOM SLAB	7.00	2.30	1	BOTTOM SLAB	6.45	
	CTOR (TT)	TNT7B	42.000		2.29	96.18	1.40	2.67	1	BOTTOM SLAB	7.00	2.29	1	BOTTOM SLAB	6.45	
	TRA(	TNAGRIT4	43.000		2.04	87.72	1.40	2.56	1	BOTTOM SLAB	7.00	2.04	1	BOTTOM SLAB	6.45	
	TRUCK	TNAGT5A	45.000		2.05	92.25	1.40	2.56	1	BOTTOM SLAB	7.00	2.05	1	BOTTOM SLAB	6.45	
	TR	TNAGT5B	45.000		2.03	91.35	1.40	2.46	1	BOTTOM SLAB	7.00	2.03	1	BOTTOM SLAB	6.45	1



ASSEMBLED BY : J.M. KEPICH CHECKED BY : R.F. WERTMAN	
DRAWN BY : WMC 7/II CHECKED BY : GM 7/II	REV.10/1/11 MAA/GM

## LRFR SUMMARY

(LOOKING DOWNSTREAM)



THESE PLANS HAVE BEEN PROPERLY EXAMINED BY THE UNDERSIGNED.I HAVE DETERMINED THAT THEY COMPLY WITH EXISTING NORTH CAROLINA CODES, AND HAVE BEEN PROPERLY ADAPTED FOR USE IN THIS AREA.

### LOAD FACTORS:

LOAD TYPE	MAX FACTOR	MIN FACTOR
DC	1.25	0.90
DW	1.50	0.65
EV	1.30	0.90
EH	1.35	0.90
ES	1.35	0.90
LS	1.75	
WA	1.00	

DESIGN LOAD RATING FACTORS

### NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

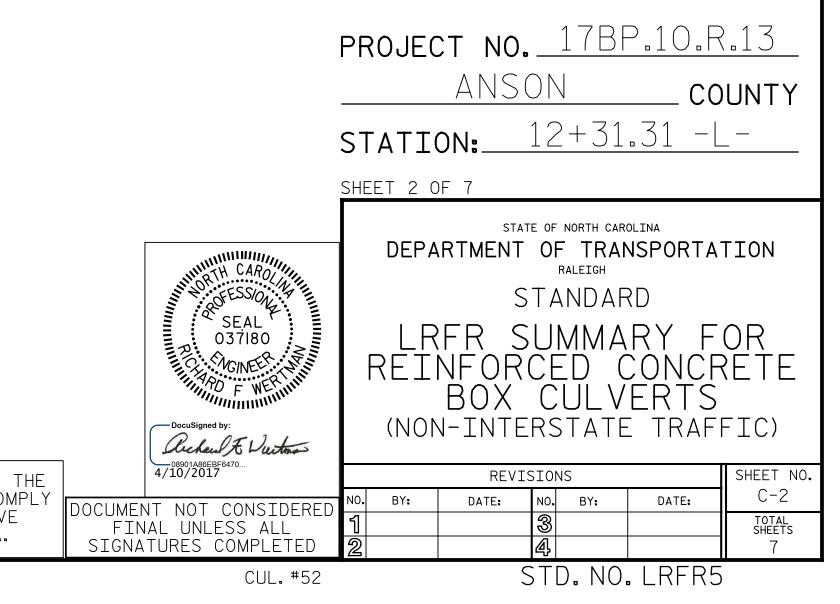
### COMMENTS:

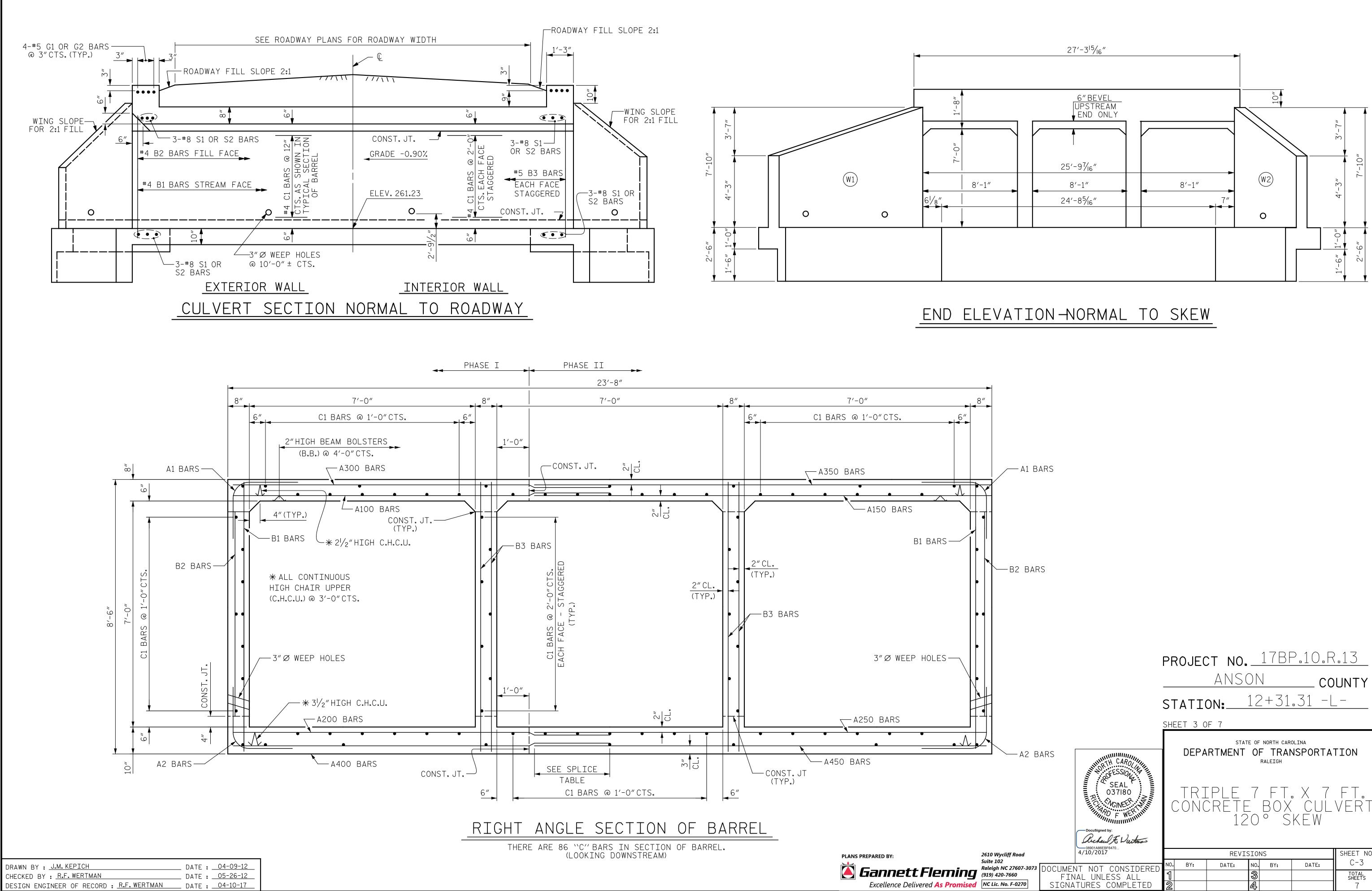
1. BOX STRUCTURE IS SYMMETRICAL, THEREFORE BOX 1 RATINGS SIMILAR TO BOX 3.

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

- 3 LEGAL LOAD RATING \*\*
- \*\* SEE CHART FOR VEHICLE TYPE





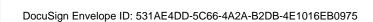
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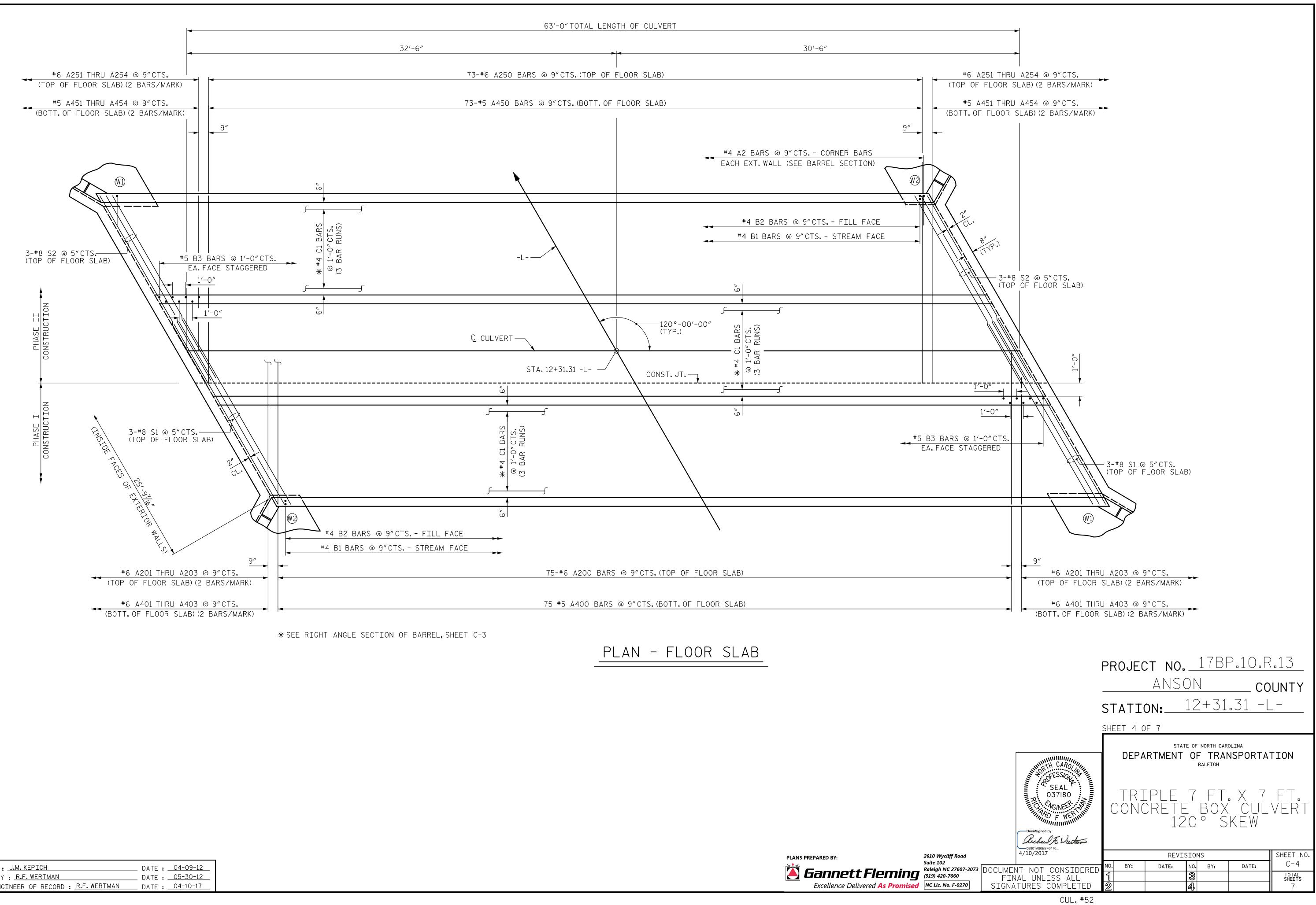
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RALEIGH RAL RALEIGH RALEIGH RALEIGH RALEIGH RALEIGH RALEIGH
ff Road DocuSigned by: Dichem For Vietna 4/10/2017 REVISIONS SHEET NO.
27607-3073DOCUMENT NOT CONSIDERED FINAL UNLESS ALLNO.BY:DATE:NO.BY:DATE:C-33FINAL UNLESS ALL13International StreetsTotal SHEETS555247

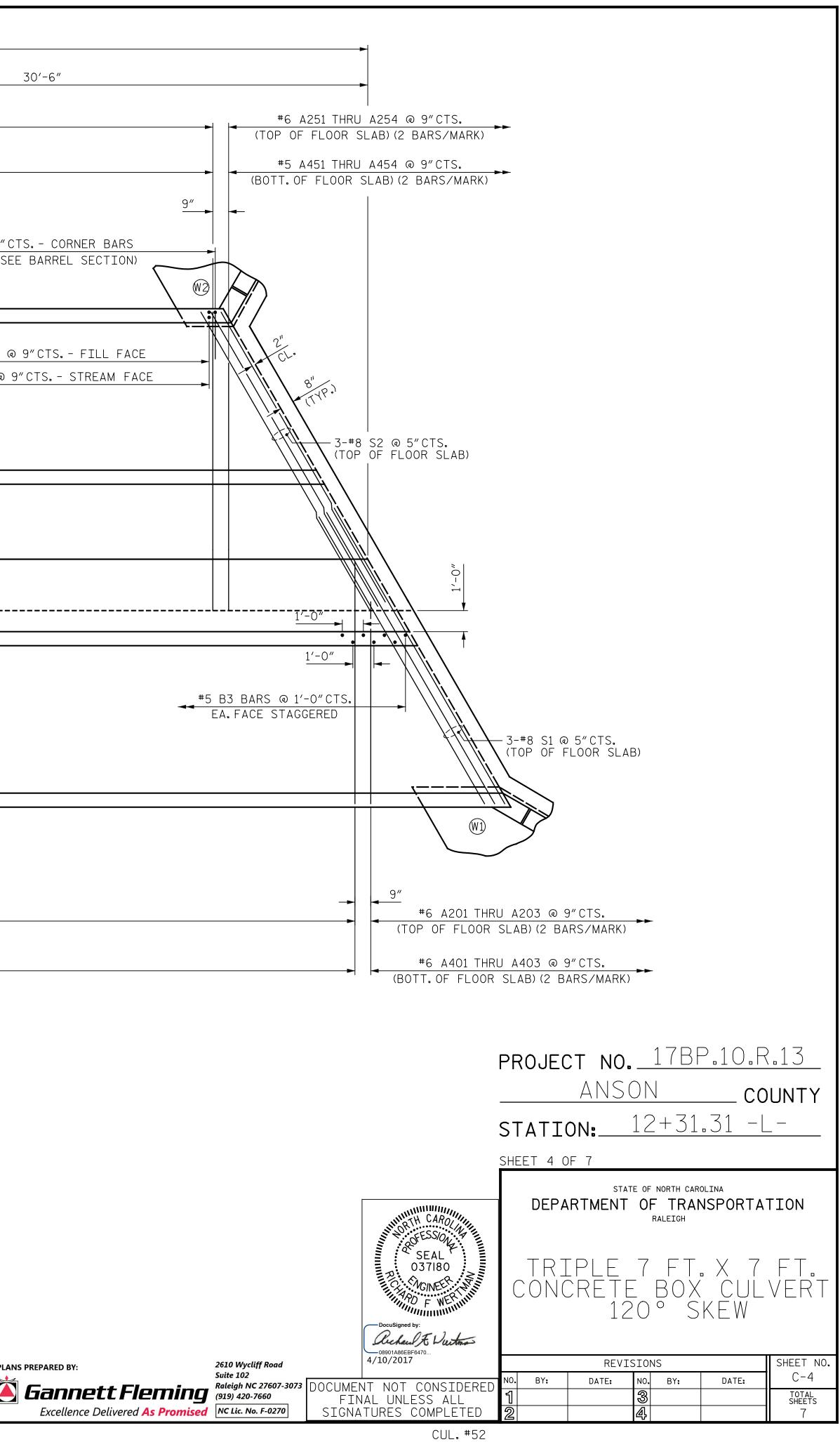
CUL.#52



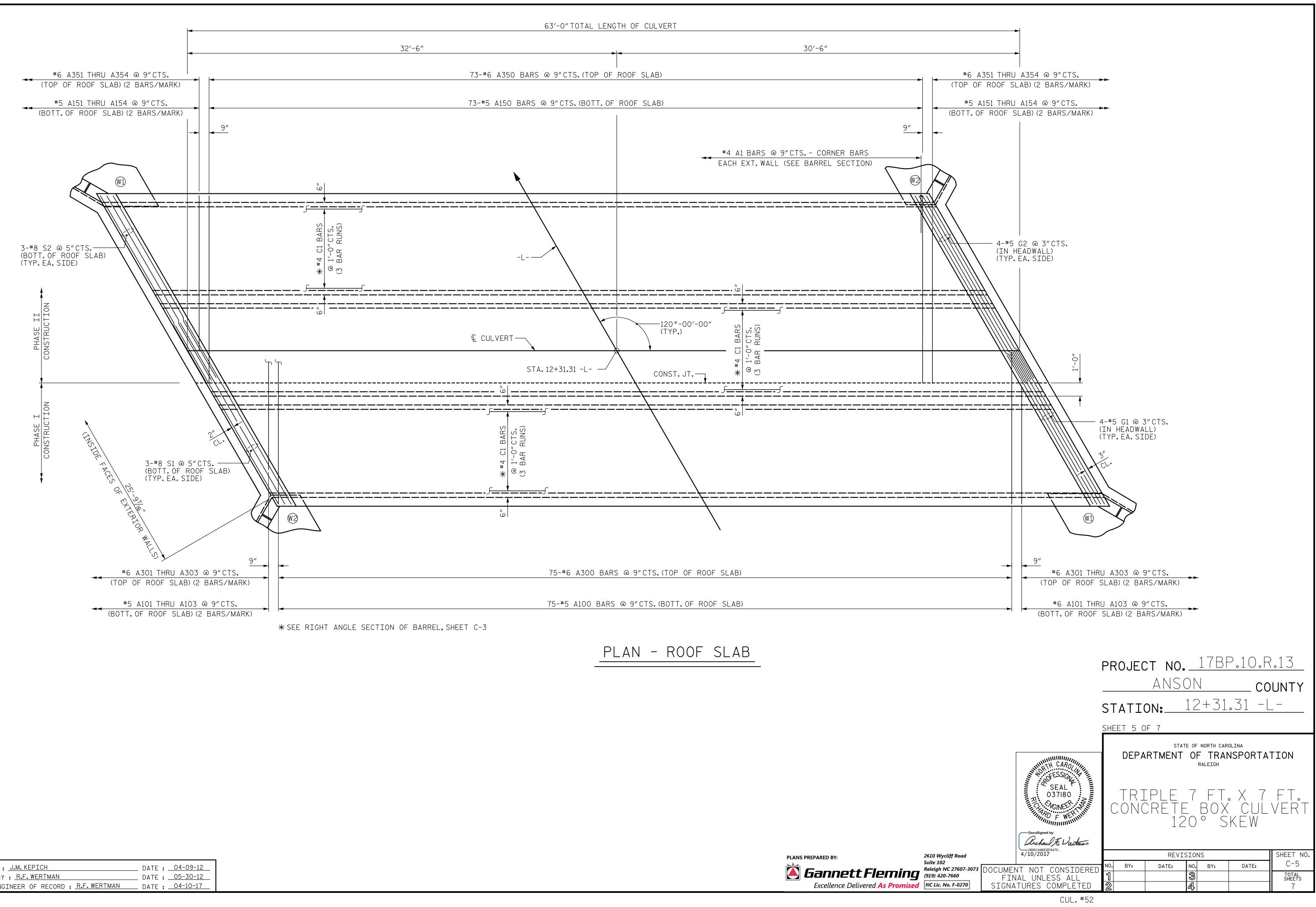
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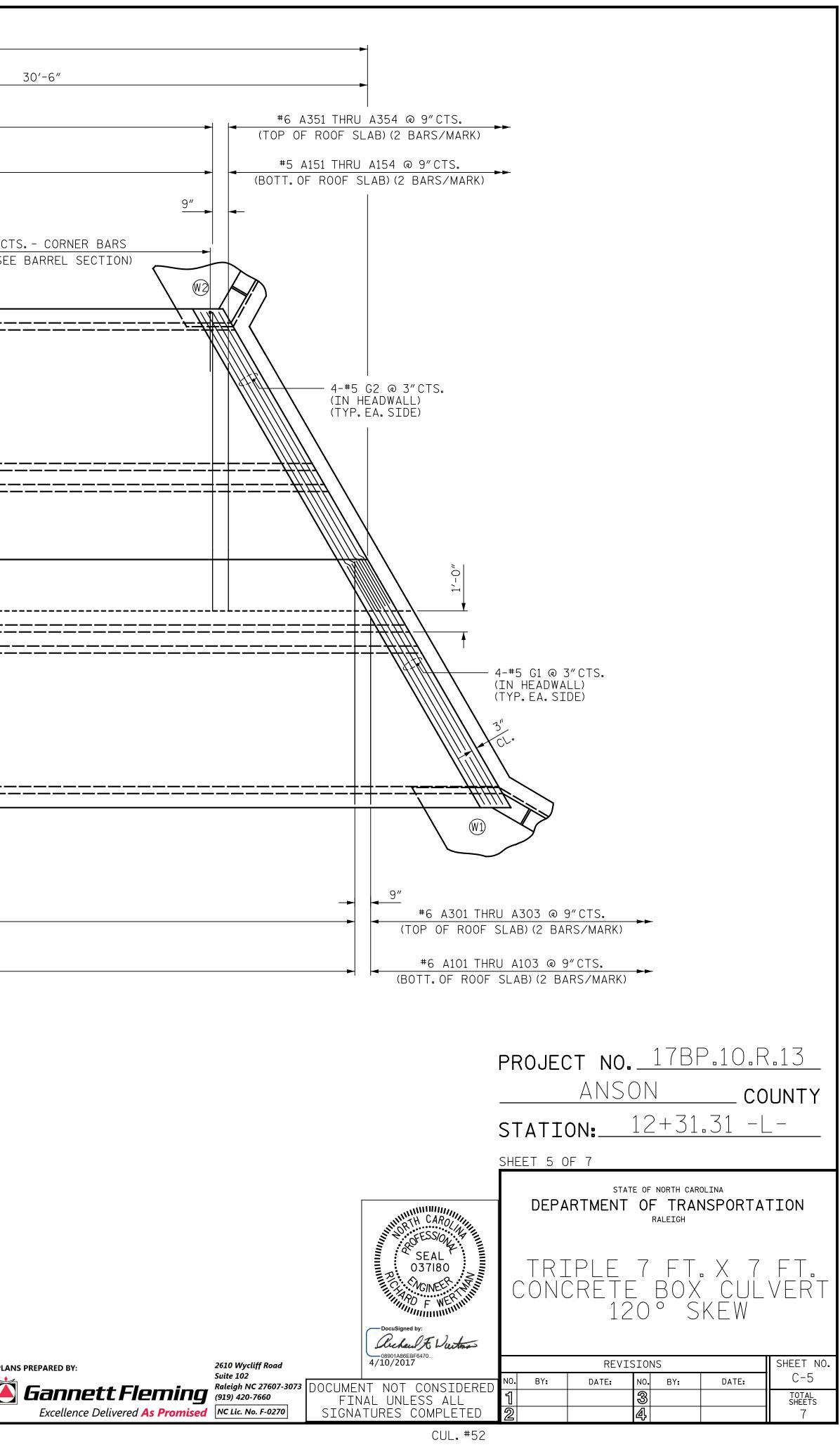
DRAWN BY : J.M. KEPICH	DATE : <u>04-09-12</u>
CHECKED BY : R.F. WERTMAN	DATE: 05-30-12
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	DATE : <u>04-10-17</u>





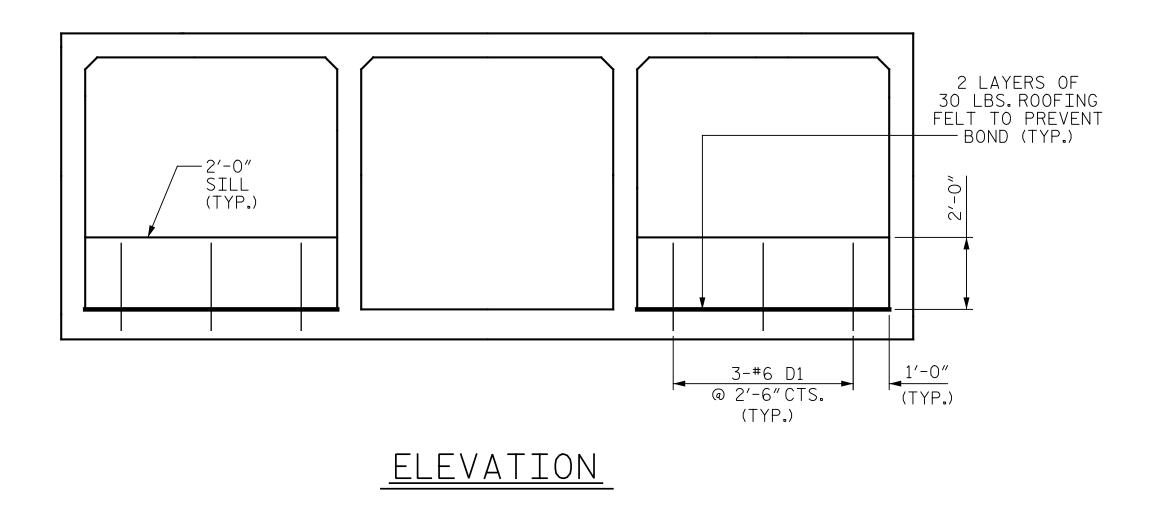


DRAWN BY : J.M. KEPICH	DATE : .	04-09-12
CHECKED BY : _R.F. WERTMAN	DATE : .	05-30-12
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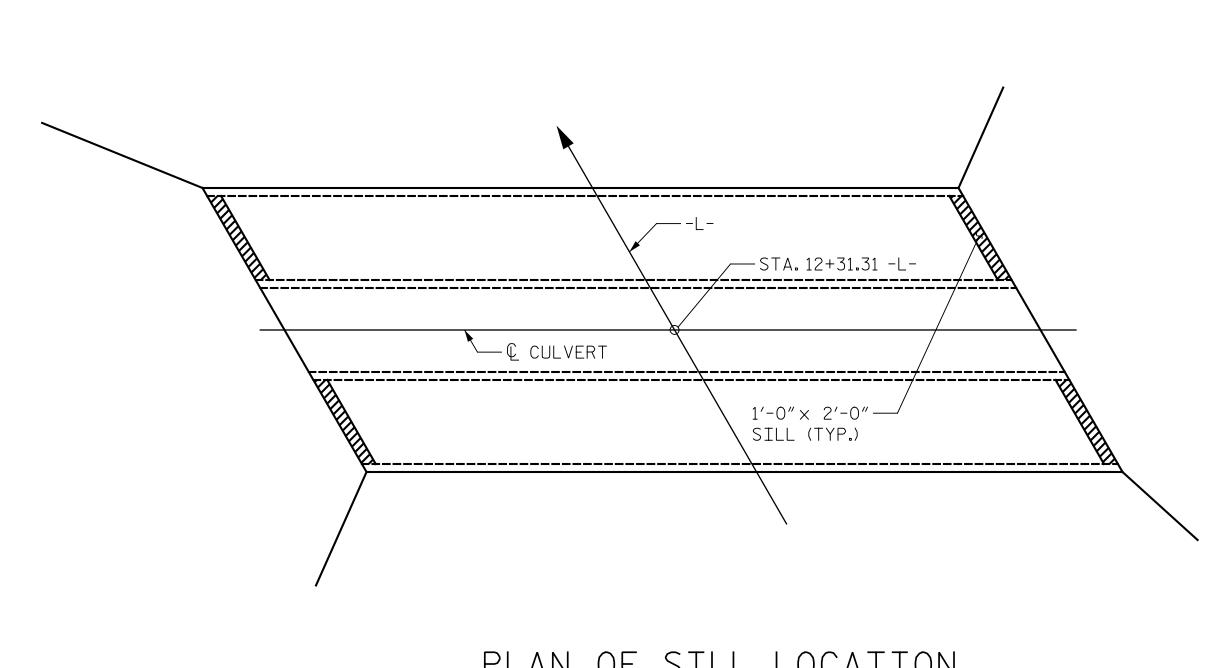


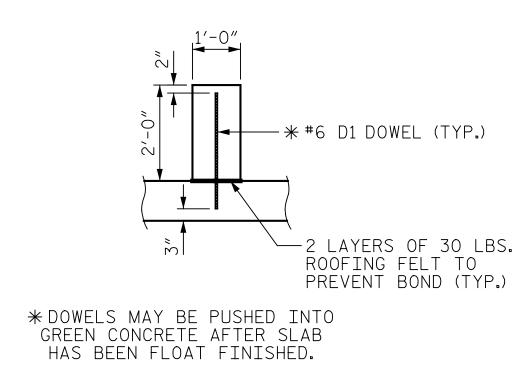
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SPLI	CE LENGT	h chart					ΒA	r sc	HED	ULE	- -			
BAR	SIZE	SPLICE LENGTH	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIG
A100	5	1'-9"	A1	164	4	3	4'-8"	511	A350	73	6	STR	14'-2"	155
A200	6	2'-3"	A2	164	4	3	4'-9"	520	A351	4	6	STR	11'-9″	71
A300	6	2'-3"							A352	4	6	STR	9'-2"	55
A400	5	1'-9"	A100	75	5	STR	10'-11"	854	A353	4	6	STR	6'-7″	40
			A101	4	5	STR	9'-8"	40	A354	4	6	STR	4'-0"	24
C1	4	1'-11"	A102	4	5	STR	7'-1″	30						
			A103	4	5	STR	4'-6"	19	A400	75	5	STR	10'-11"	854
"G "	5	2'-5"							A401	4	5	STR	9'-8"	40
			A150	73	5	STR	14'-2"	1079	A402	4	5	STR	7'-1″	30
"S"	8	4'-11"	A151	4	5	STR	11'-9″	49	A403	4	5	STR	4'-6"	19
			A152	4	5	STR	9'-2"	38						
	BAR TYP	FS	A153	4	5	STR	6'-7"	27	A450	73	5	STR	14'-2"	107
			A154	4	5	STR	4'-0"	17	A451	4	5	STR	11'-9″	49
									A452	4	5	STR	9'-2″	38
	$\frown$		A200	75	6	STR	11′-5″	1286	A453	4	5	STR	6'-7″	27
	(3)		A201	4	6	STR	9'-8"	58	A454	4	5	STR	4'-0"	17
	$\bigcirc$	A1 A2	A202	4	6	STR	7'-1″	43						
			A203	4	6	STR	4'-6"	27	B1	168	4	STR	8'-0"	898
	VERTICAL LEG	- 7 / 1							B2	168	4	STR	6'-4″	711
		5	A250	73	6	STR	14'-2"	1553	B3	254	5	STR	8'-0"	211
	6" RAD.		A251	4	6	STR	11'-9″	71						
			A252	4	6	STR	9'-2"	55	C1	258	4	STR	22'-2"	382
		, "	A253	4	6	STR	6'-7"	40						
		3/2	A254	4	6	STR	4'-0"	24	D1	12	6	STR	2'-5"	44
	A1 1'-9 <sup>1</sup> /2"													
	A2 1'-9 <sup>1</sup> /2"		A300	75	6	STR	11'-5″	1286	G1	8	5	STR	13'-0"	108
			A301	4	6	STR	9'-8"	58	G2	8	5	STR	16'-4"	136
			A302	4	6	STR	7'-1"	43						
ALL BAR D	IMENSIONS ARE O	UT TO OUT.	A303	4	6	STR	4'-6"	27	S1	12	8	STR	15′-6″	49
									S2	12	8	STR	16'-4"	52
									REINF	ORCIN	NG STE	EL	20,5	07 LB



DRAWN BY : J.M. KEPICH	DATE :_	04-09-12
CHECKED BY : R.F. WERTMAN	DATE :_	05-30-12
DESIGN ENGINEER OF RECORD : R.F. WERTMAN	DATE :_	04-10-17





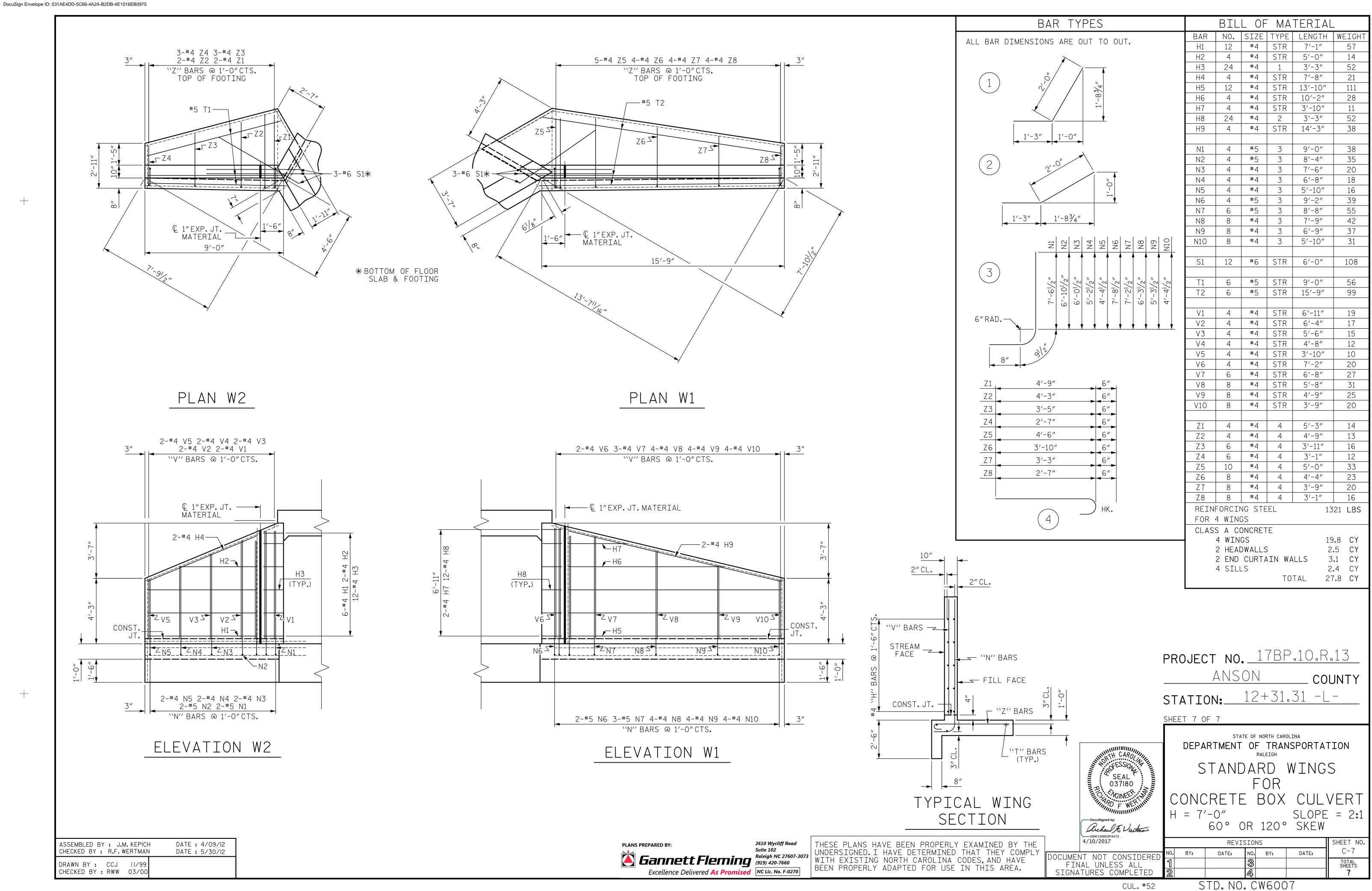
SECTION THRU 2'-O"SILL



## PLAN OF SILL LOCATION

				CT NO. <u>ANS(</u> ON:			CO	UNTY
		SH	EET 6 C	)F 7				
	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					TION		
	ROFESSION AND SEAL 037180 F WERNIN	(	TRIPLE 7 FT.X 7 FT. Concrete box culvert 120° skew					
	DocuSigned by: Dechaul F. Vuitoro 08901A86EBF6470							
f Road	4/10/2017		REVISIONS SHEET NO.					
		NO.	BY:	DATE:	NO.	BY:	DATE:	C-6
27607-3073 DOCUME 560 FT	ENT NOT CONSIDEREI NAL UNLESS ALL	1			3			TOTAL SHEETS
	ATURES COMPLETED	2			4			7

CUL.#52



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	BIL	L OF	MA	TERIAL	_			
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT			
H1	12	#4	STR	7'-1″	57			
H2	4	#4	STR	5′-0″	14			
H3	24	#4	1	3'-3"	52			
Η4	4	#4	STR	7'-8″	21			
H5	12	#4	STR	13′-10″	111			
H6	4	#4	STR	10'-2"	28			
Η7	4	#4	STR	3′-10″	11			
H8	24	#4	2	3'-3"	52			
H9	4	#4	STR	14'-3″	38			
N1	4	#5	3	9'-0"	38			
N2	4	#5	3	8'-4"	35			
Ν3	4	#4	3	7'-6″	20			
N4	4	#4	3	6'-8″	18			
N5	4	#4	3	5'-10"	16			
N6	4	#5	3	9'-2"	39			
N7	6	#5	3	8'-8"	55			
N8	8	#4	3	7′-9″	42			
N9	8	#4	3	6'-9"	37			
N10	8	#4	3	5'-10"	31			
S1	12	#6	STR	6'-0"	108			
Τ1	6	#5	STR	9'-0"	56			
Τ2	6	#5	STR	15'-9″	99			
V1	4	#4	STR	6'-11″	19			
V2	4	#4	STR	6'-4"	17			
٧3	4	#4	STR	5′-6″	15			
V4	4	#4	STR	4'-8"	12			
V5	4	#4	STR	3'-10"	10			
V6	4	#4	STR	7'-2″	20			
V7	6	#4	STR	6'-8″	27			
V8	8	#4	STR	5′-8″	31			
V9	8	#4	STR	4'-9"	25			
V10	8	#4	STR	3'-9"	20			
71	<u>л</u>	<u></u> н л	Λ		1 1			
Z1	4	#4	4	5'-3"	14			
Z2	4	#4	4	4'-9"	13			
Z3	6	#4 #4	4	3'-11"	16			
Z4	6	#4 #4	4	3'-1"	12			
Z5	10	#4	4	5'-0"	33			
Z6	8	#4	4	4'-4"	23			
Z7	8	#4 #4	4	3'-9"	20			
Z8		#4	4	3'-1"	16			
		NG STE	EL	13	321 LBS			
	4 WING		· –-					
			F	1 ~				
4 WINGS 19.8 CY								
2 HEADWALLS 2.5 CY 2 END CURTAIN WALLS 3.1 CY								
4 SILLS 2.4 CY								
	I JILL		Τ∩		.4 CT 7.8 CY			
			10					

#### DESIGN DATA:

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SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.
	(MINIMUM)

#### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

#### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS. ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

#### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS. SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

## STANDARD NOTES

### ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION. VERTICAL CURVE ORDINATE. AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK. THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

#### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS. WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE

INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE  $\frac{3}{4}$ " Ø studs specified on the plans. This substitution shall be made at THE RATE OF 3 - 7/8″Ø STUDS FOR 4 - 3/4″Ø STUDS,AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16"IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING. GALVANIZING. OR METALLIZING.

### HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

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